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SEAL

ISSUE	BY	DATE	DESCRIPTION
-		08/27/24	PERMIT SET

PROJECT INFORMATION BLOCK	
JOB #	230646
DATE	08/27/24
DRAWN BY:	SSC
CHECKED BY:	JDP

SHEET TITLE  
**COMPONENTS & CLADDING**  
 SHEET NUMBER

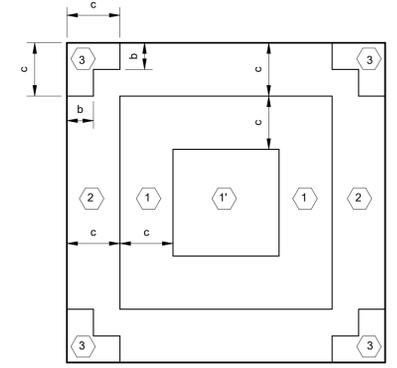
**Abbreviations**

@	AT	EF	EACH FACE	NWC	NORMAL WEIGHT CONCRETE
&	AND	EJ	EXPANSION JOINT	NIC	NOT IN CONTRACT
#	NUMBER OR POUND	ES	EACH SIDE	NS	NS
%	PERCENT	EQ	EQUAL	N.T.S	NOT TO SCALE
Ø	DIAMETER	EXP	EXPANSION	OC	ON CENTER
°	DEGREE	EW	EACH WAY	OPNG	OPENING
		EOD	EDGE OF DECK	OPP	OPPOSITE
		EOS	EDGE OF SLAB	PC	PILE CAP
ASD	ALLOWABLE STRESS DESIGN	ELEC	ELECTRICAL	PL	PLATE
ACI	AMERICAN CONCRETE INSTITUTE	EMBED	ELECTRICAL	PSF	POUNDS PER SQUARE FOOT
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	ENGR	ENGINEER	PSI	POUNDS PER SQUARE INCH
AISI	AMERICAN IRON AND STEEL INSTITUTE	FF	FINISH FACE	PCI	POUNDS PER CUBIC FOOT
ASTM	AMERICAN SOCIETY FOR TESTING MATERIALS	FTG	FOOTING	PLBG	PLUMBING
AWS	AMERICAN WELDING SOCIETY	FDN	FOUNDATION	PLF	POUNDS PER LINEAR FOOT
AFF	ABOVE FINISH FLOOR	FV	FIELD VERIFY	PT	POST-TENSIONING
ARCH.	ARCHITECT OR ARCHITECTURAL	FS	FAR SIDE	R	RADIUS
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL			REIN	REINFORCEMENT
AHU	AIR HANDLING UNIT	GALV	GALVANIZED	SECT	SECTION
ALT	ALTERNATE	GA	GAUGE	SC	SHEAR CONNECTOR
APPROX	APPROXIMATE	GR	GRADE	SHT	SHEET
APPRV	APPROVED	H. STUD	HEADED STUDS	SLV	SHORT LEG VERTICAL
				SIM	SIMILAR
B	BOTTOM			SOG	SLAB ON GRADE
BRG	BEARING	IN	INCH	STD	STANDARD
BOF	BOTTOM OF FOUNDATION	INV	INVERT	STL	STEEL
BOT	BOTTOM	INFO	INFORMATION	SDI	STEEL DECK INSTITUTE
BLDG	BUILDING	JT	JOINT	SF	SQUARE FOOT
BTWN	BETWEEN	JST	JOIST	STIFF	STIFFENER
		JG	JOIST GIRDER	SUP	SUPPORT
CIP	CAST-IN-PLACE			SYM	SYMMETRICAL
CONC	CONCRETE	K	KIP (1000 LBS)	STD	STANDARD
CMU	CONCRETE MASONRY UNIT	KSF	KIPS PER SQUARE FOOT		
CRSI	CONCRETE REINFORCING STEEL INSTITUTE	KSI	KIPS PER SQUARE INCH	T	TOP
CONT	CONTINUOUS	KLF	KIPS PER LINEAR FOOT	TO	TOP OF
CJ	CONTROL JOINT	LW	LIGHT WEIGHT	TOC	TOP OF CONCRETE
CLR	CLEAR	LWC	LIGHT WEIGHT CONCRETE	TOF	TOP OF FOOTING
COL	COLUMN	LRFD	LOAD AND RESISTANCE FACTOR DESIGN	TOS	TOP OF STEEL
C	CHANNEL			TOW	TOP OF WALL
COMP	COMPRESSION	LLV	LONG LEG VERTICAL	TOPC	TOP OF PILE CAP
CONN	CONNECTION	LLH	LONG LEG HORIZONTAL	TYP	TYPICAL
CONTR	CONTRACTOR	LBS	POUNDS	T	TENSION
		L	ANGLE	UNO	UNLESS NOTED OTHERWISE
DIA	DIAMETER	MTL	METAL	US	UNDERSIDE
DIM	DIMENSION	MIN	MINIMUM		
DWLS	DOWELS	MAX	MAXIMUM		
DBL	DOUBLE	MRF	MANUFACTURER	WWR	WELDED WIRE REINFORCEMENT
DEG	DEGREE	M	MOMENT	W	WITH
DP	DRILLED PIER	MECH	MECHANICAL	WP	WORK POINT
EA	EACH	MISC	MISCELLANEOUS	W/O	WITHOUT
EE	EACH END				

Flat Roof Ultimate Wind Pressures		
Area (sq.ft)	Max + Pressure (psf)	Max - Pressure (psf)
Interior Zone 1'		
10	16.0	-33.2
20	16.0	-33.2
50	16.0	-33.2
≥100	16.0	-33.2
Interior Zone 1		
10	16.0	-57.8
20	16.0	-54.0
50	16.0	-48.9
≥100	16.0	-45.1
Perimeter Zone 2		
10	33.2	-76.2
20	31.7	-71.3
50	29.8	-64.8
≥100	28.3	-59.9
Corner Zone 3		
10	33.2	-103.9
20	31.7	-94.1
50	29.8	-81.1
≥100	28.3	-71.3

h = 14' - 0"  
 b = 0.2h = 2' - 10"  
 c = 0.6h = 8' - 5"

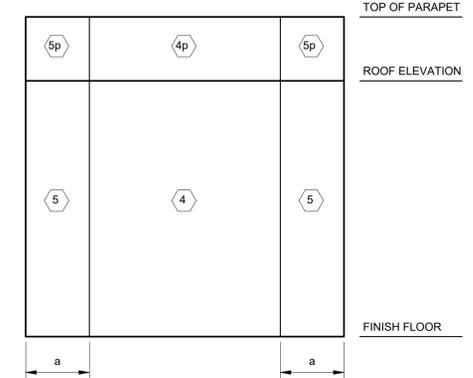
RELIABLE DEAD LOAD FOR UPLIFT ON ROOF STRUCTURE = 6 PSF



**1 Flat Roof Components and Cladding Wind Pressures**  
 N.T.S.

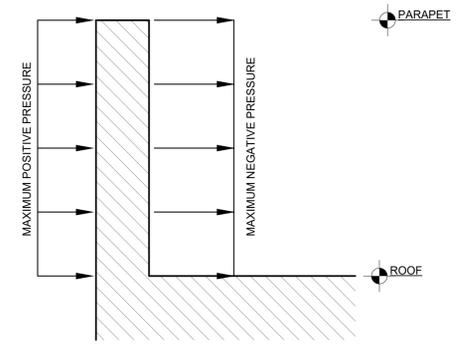
Wall Ultimate Wind Pressures		
Area (sq.ft)	Max + Pressure (psf)	Max - Pressure (psf)
Interior Zone 4		
10	33.2	-36.0
20	31.7	-34.5
50	29.8	-32.5
100	28.3	-31.1
≥200	24.9	-27.7
Corner Zone 5		
10	33.2	-44.2
20	31.7	-41.3
50	29.8	-37.4
100	28.3	-34.5
≥200	24.9	-27.7

a = 4' - 11"



**2 Wall Components and Cladding Wind Pressures**  
 N.T.S.

Parapet Ultimate Wind Pressures (Windward + Leeward)	
Area (SQ.FT)	Max + Pressure (PSF)
Interior Zone 4p	
10	104.2
20	97.5
50	88.6
100	81.8
Corner Zone 5p	
10	104.2
20	97.5
50	88.6
100	81.8



NOTES:  
 1. IN DESIGN OF METAL STUD PARAPETS, ADD ABSOLUTE VALUES OF MAXIMUM POSITIVE AND NEGATIVE PARAPET PRESSURES IN APPLICABLE ZONE.  
 2. IN DESIGN OF EXTERIOR FINISHES ATTACHMENT, USE THE LARGEST ABSOLUTE VALUE OF LISTED PRESSURES FOR THE APPLICABLE ZONE.

**3 Parapet Components and Cladding Wind Pressures**  
 N.T.S.

SYMBOL	DESCRIPTION
	NEW COLUMN GRID LINE
	EXISTING COLUMN GRID LINE
	PLAN TITLE
	SECTION TITLE
	ELEVATION TAG
	SECTION TAG

Symbol Legend  
 N.T.S.



## Structural Steel

NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	INSPECTOR SHALL BE ON THE PREMISES FOR INSPECTION DURING THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL		AISC 360 SEC. N5.7	ATA
1.01	DIAMETER, GRADE, TYPE, LENGTH, AND EMBEDMENT DEPTH OF ANCHOR RODS AND OTHER EMBEDDED ITEMS	PERFORM		ATA
1.02	INSPECT THE FABRICATED STEEL OR ERECTED STEEL FRAME, AS APPROPRIATE, TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION.	PERFORM		ATA
2.00	INSPECTION TASKS PRIOR TO WELDING:		AISC 360 SEC. N5.4	
2.01	WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE	PERFORM		ATA
2.02	MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	PERFORM		ATA
2.03	MATERIAL IDENTIFICATION (TYPE/GRADE)	OBSERVE		ATA
2.04	WELDER IDENTIFICATION SYSTEM (a)	OBSERVE		ATA
2.05	FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY), JOINT PREPARATION, DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL), CLEANLINESS (CONDITION OF STEEL SURFACES), TACKING (TACK WELD QUALITY AND LOCATION), AND BACKING TYPE AND FIT (IF APPLICABLE)	OBSERVE		ATA
2.06	CONFIGURATION AND FINISH OF ACCESS HOLES	OBSERVE		ATA
2.07	CHECK WELDING EQUIPMENT	OBSERVE		ATA
3.00	INSPECTION TASKS DURING WELDING:		AISC 360 SEC. N5.4	
3.01	USE OF QUALIFIED WELDERS	OBSERVE		ATA
3.02	CONTROL AND HANDLING OF WELDING CONSUMABLE PACKAGING AND EXPOSURE CONTROL	OBSERVE		ATA
3.03	NO WELDING OVER CRACKED TACK WELDS	OBSERVE		ATA
3.04	ENVIRONMENTAL CONDITIONS INCLUDING WIND SPEED WITHIN LIMITS, PRECIPITATION, AND TEMPERATURE	OBSERVE		ATA
3.05	WPS FOLLOWED INCLUDING SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, SELECTED WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED (MIN/MAX), AND PROPER POSITION (F, V, H, OH)	OBSERVE		ATA
3.06	WELDING TECHNIQUES INCLUDING: INTERPASS AND FINAL CLEANING, EACH PASS WITHIN PROFILE LIMITATIONS, EACH PASS MEETS QUALITY REQUIREMENTS	OBSERVE		ATA
4.00	INSPECTION TASKS AFTER WELDING:		AISC 360 SEC. N5.4	
4.01	WELDS CLEANED	OBSERVE		ATA
4.02	SIZE, LENGTH, AND LOCATION OF WELDS	PERFORM		ATA
4.03	WELDS MEET VISUAL ACCEPTANCE CRITERIA FOR: CRACK PROHIBITION, WELD/BASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT, AND POROSITY	PERFORM		ATA
4.04	ARC STRIKES	PERFORM		ATA
4.05	K-AREA (b)	PERFORM		ATA
4.06	BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	PERFORM		ATA
4.07	REPAIR ACTIVITIES	PERFORM		ATA
5.00	INSPECTION TASKS PRIOR TO BOLTING:		AISC 360 SEC. N5.6	
5.01	MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	PERFORM		ATA
5.02	FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	OBSERVE		ATA
5.03	PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)	OBSERVE		ATA
5.04	PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	OBSERVE		ATA
5.05	CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	OBSERVE		ATA
5.06	PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	OBSERVE		ATA
5.07	PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	OBSERVE		ATA
6.00	INSPECTION TASKS DURING BOLTING:		AISC 360 SEC. N5.6	
6.01	FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED	OBSERVE		ATA
6.02	JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	OBSERVE		ATA
6.03	FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	OBSERVE		ATA
6.04	FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	OBSERVE		ATA
7.00	INSPECTION TASKS AFTER BOLTING:		AISC 360 SEC. N5.6	
7.01	DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	PERFORM		ATA

**NOTES:** (a) THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE.  
 (b) WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. (75MM) OF THE WELD.

## Steel Deck

NO.	INSPECTION TASK	FREQUENCY	REFERENCE FOR CRITERIA	AGENT
1.00	INSPECTION OR EXECUTION TASKS PRIOR TO DECK PLACEMENT:		SDI STD QA/QC TABLE 1.1	
1.01	VERIFY COMPLIANCE OF MATERIALS (DECK AND ALL DECK ACCESSORIES) WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES, AND BASE METAL THICKNESS.	PERFORM		ATA
1.02	DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES.	PERFORM		ATA
2.00	INSPECTION OR EXECUTION TASKS AFTER DECK PLACEMENT:		SDI STD QA/QC TABLE 1.2	
2.01	VERIFY COMPLIANCE OF DECK AND ALL DECK ACCESSORIES INSTALLATION WITH CONSTRUCTION DOCUMENTS.	PERFORM		ATA
2.02	VERIFY DECK MATERIALS ARE REPRESENTED BY THE MILL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS.	PERFORM		ATA
2.03	DOCUMENT ACCEPTANCE OR REJECTION OF INSTALLATION OF DECK AND DECK ACCESSORIES.	PERFORM		ATA
3.00	INSPECTION OR EXECUTION TASKS PRIOR TO WELDING:		SDI STD QA/QC TABLE 1.3	
3.01	WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE.	OBSERVE		ATA
3.02	MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE.	OBSERVE		ATA
3.03	MATERIAL IDENTIFICATION (TYPE/GRADE).	OBSERVE		ATA
3.04	CHECK WELDING EQUIPMENT.	OBSERVE		ATA
4.00	INSPECTION OR EXECUTION TASKS DURING WELDING:		SDI STD QA/QC TABLE 1.4	
4.01	USE QUALIFIED WELDERS.	OBSERVE		ATA
4.02	CONTROL AND HANDLING OF WELDING CONSUMABLES.	OBSERVE		ATA
4.03	ENVIRONMENTAL CONDITIONS (WIND SPEED, MOISTURE, TEMPERATURE).	OBSERVE		ATA
4.04	WPS FOLLOWED.	OBSERVE		ATA
5.00	INSPECTION OR EXECUTION TASKS AFTER WELDING:		SDI STD QA/QC TABLE 1.5	
5.01	VERIFY SIZE AND LOCATION OF WELDS, INCLUDING SUPPORT, SIDELAP, AND PERIMETER WELDS.	PERFORM		ATA
5.02	WELDS MEET VISUAL ACCEPTANCE CRITERIA.	PERFORM		ATA
5.03	VERIFY REPAIR ACTIVITIES.	PERFORM		ATA
5.04	DOCUMENT ACCEPTANCE OR REJECTION OF WELDS.	PERFORM		ATA
6.00	INSPECTION OR EXECUTION TASKS PRIOR TO MECHANICAL FASTENING:		SDI STD QA/QC TABLE 1.6	
6.01	MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS.	OBSERVE		ATA
6.02	PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION.	OBSERVE		ATA
6.03	PROPER STORAGE FOR MECHANICAL FASTENERS.	OBSERVE		ATA
7.00	INSPECTION OR EXECUTION TASKS DURING MECHANICAL FASTENING:		SDI STD QA/QC TABLE 1.7	
7.01	FASTENERS ARE POSITIONED AS REQUIRED.	OBSERVE		ATA
7.02	FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.	OBSERVE		ATA
8.00	INSPECTION OR EXECUTION TASKS AFTER MECHANICAL FASTENING:		SDI STD QA/QC TABLE 1.8	
8.01	CHECK SPACING, TYPE, AND INSTALLATION OF SUPPORT FASTENERS.	PERFORM		ATA
8.02	CHECK SPACING, TYPE, AND INSTALLATION OF SIDELAP FASTENERS.	PERFORM		ATA
8.03	CHECK SPACING, TYPE, AND INSTALLATION OF PERIMETER FASTENERS.	PERFORM		ATA
8.04	VERIFY REPAIR ACTIVITIES.	PERFORM		ATA
8.05	DOCUMENT ACCEPTANCE OR REJECTION OF MECHANICAL FASTENERS.	PERFORM		ATA



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**FIFTH THIRD**  
**ORANGE BLOSSOM**  
 NWC OIL WELL ROAD & BIG  
 CORKSCREW DRIVE  
 NAPLES, FLORIDA 34120

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SHEET TITLE  
**BRANCH FOUNDATION AND FLOOR PLAN**

SHEET NUMBER

S-201



**1 Branch Foundation and Floor Plan**  
 1/4" = 1'-0"

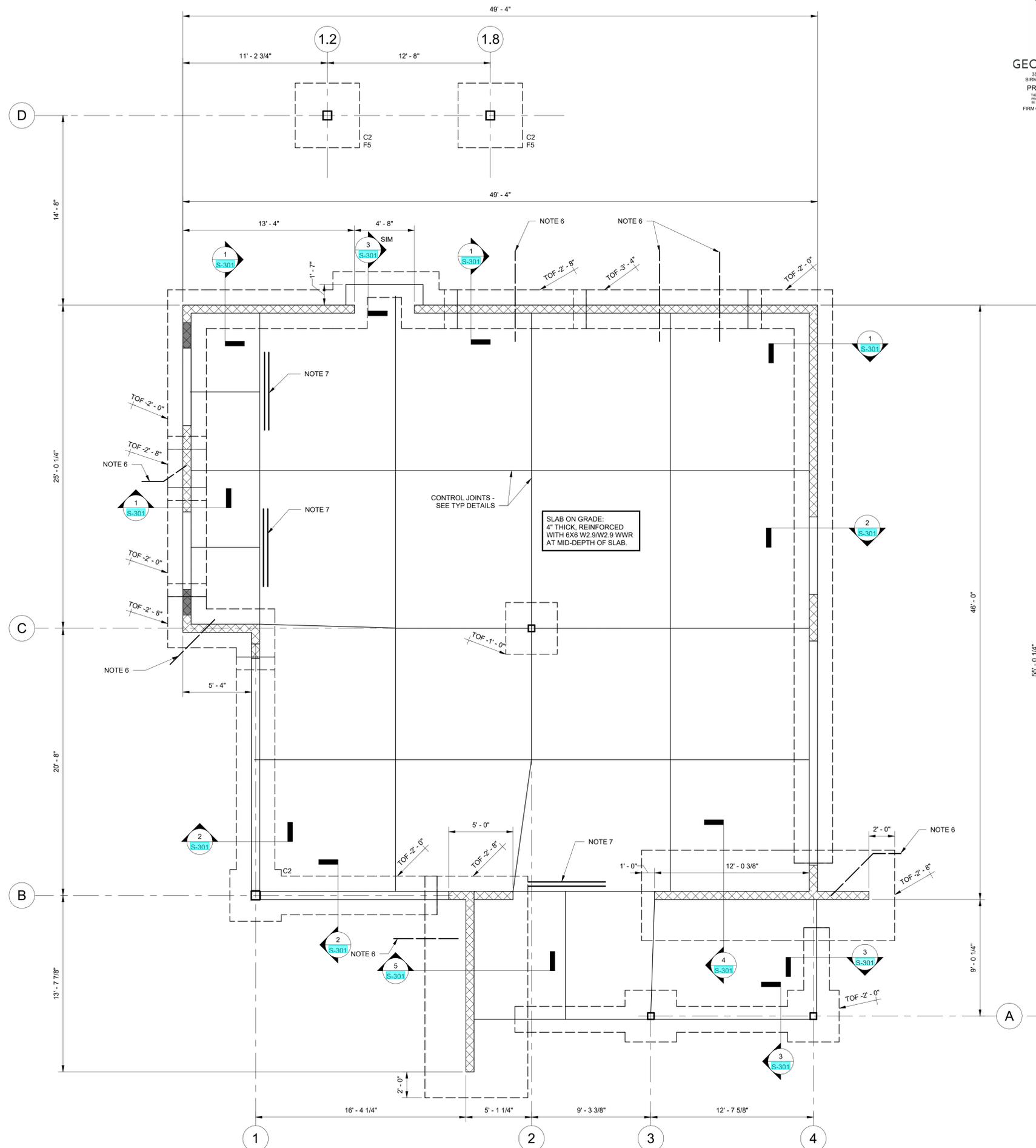
1. FINISH FLOOR (TOP OF SLAB) ELEVATION 0'-0", UNLESS NOTED.
2. TOP OF FOOTING: -2'-0", UNLESS NOTED.
3. FOR SLAB ON GRADE CONSTRUCTION, SEE GENERAL NOTES AND TYPICAL DETAILS. FOR SLAB EDGES NOT DIMENSIONED, SEE ARCHITECTURAL DRAWINGS.
4. FOR SLAB RECESS, SEE ARCHITECTURAL DRAWINGS.
5. GENERAL CONTRACTOR SHALL COORDINATE TILE JOINT LOCATIONS WITH CONTROL JOINTS.
6. FOOTING STEP ELEVATIONS AND LOCATION ARE APPROXIMATE. GENERAL CONTRACTOR SHALL COORDINATE ALL FOOTING STEPS WITH CIVIL, PLUMBING AND UTILITY DRAWINGS.
7. PROVIDE 2#5x6'-0" AT ALL RE-ENTRANT CORNERS AND DISCONTINUOUS CONTROL JOINTS.
8. PROVIDE ISOLATION JOINT AROUND COLUMN THROUGH SLAB AND CURB, TYPICAL AT DRIVE-THRU.
9. BUILD MASONRY CONNECTION ACROSS WALLS AT THREE LOCATIONS.
10. AT BEAM BEARING ABOVE, PROVIDE (3) CELLS REINFORCED WITH 1 #5 EACH AND GROUTED SOLID FROM BEAM BEARING TO FOUNDATION.
11. CONTRACTOR TO COORDINATE PIPE AND CONDUIT LOCATIONS IN CMU WALLS TO BE IN UNREINFORCED CELLS ONLY.

Structural Column Schedule				
Designation	Size	Base Plate	Anchor Rods	Notes
C1	HSS6x6x3/8	3/4x12x1'-0"	(4) 3/4"Ø, 9" EMBEDMENT	
C2	HSS6x6x3/8	3/4x14x1'-2"	(4) 3/4"Ø, 9" EMBEDMENT	

1. ALL COLUMNS ARE C1, UNLESS NOTED.
2. PROVIDE 1 1/2" EDGE DISTANCE FOR ANCHOR RODS.

Structural Foundation Schedule						
Designation	Length	Width	Depth	Bottom Reinforcement	Top Reinforcement	Notes
F4	4'-0"	4'-0"	1'-2"	5#5 EW	5#5 EW	
F5	5'-0"	5'-0"	1'-0"	6#5 EW	6#5 EW	

1. ALL FOOTINGS ARE F4, UNLESS NOTED.





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 NAPLES, FLORIDA 34120

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PROJECT INFORMATION BLOCK	
JOB #	230646
DATE	08/27/24
DRAWN BY:	SSC
CHECKED BY:	JDP

SHEET TITLE  
**BRANCH ROOF FRAMING PLAN**

SHEET NUMBER

S-211



**1 Branch Roof Framing Plan**  
 1/4" = 1'-0"

- TOP OF STEEL (JOIST BEARING) ELEVATION: SEE PLAN.
- ROOF SYSTEM: 1 1/2" ROOF DECK ON STEEL JOISTS AND BEAMS. SEE GENERAL NOTES.
- TOP OF STEEL IS EITHER LEVEL OR SLOPING UNIFORMLY BETWEEN NOTED ELEVATIONS.
- BEAMS PARALLEL TO JOISTS ARE 2 1/2" HIGHER THAN SUPPORTING MEMBERS.
- ALL JOIST BEARING DEPTHS ARE 2 1/2", UNLESS NOTED.
- SPACE STEEL JOISTS EQUALLY BETWEEN BEAMS OR COLUMN LINES UNLESS NOTED.
- EQUIPMENT LOCATIONS AND WEIGHTS SHOWN ARE APPROXIMATE. THE GENERAL CONTRACTOR SHALL COORDINATE AND VERIFY THE SIZE, WEIGHT AND LOCATION OF ALL MECHANICAL UNITS WITH THE JOIST MANUFACTURER. DO NOT SCALE FROM THESE DRAWINGS. ALL NEW ROOFTOP EQUIPMENT TO BE CURB MOUNTED AND TIED DOWN PER MANUFACTURER'S INSTALLATION REQUIREMENTS. SEE TYPICAL DETAILS FOR ADDITIONAL INFORMATION.
- HANGER LOCATIONS FOR PIPING LARGER THAN 3 INCHES IN DIAMETER MUST BE COORDINATED BY THE GENERAL CONTRACTOR WITH THE JOIST MANUFACTURER. FOR WEIGHT OF PIPING AND ANY ADDITIONAL JOIST REINFORCING, SEE TYPICAL DETAIL.
- OPENINGS IN MASONRY BOND BEAMS AND LINTELS ARE NOT PERMITTED, UNLESS SPECIFICALLY INDICATED ON STRUCTURAL DRAWINGS.
- AT BEAM BEARING, PROVIDE (3) CELLS REINFORCED WITH 1 #5 EACH AND GROUTED SOLID FROM BEAM BEARING TO FOUNDATION.
- CONTRACTOR TO COORDINATE PIPE AND CONDUIT LOCATIONS IN CMU WALLS TO BE IN UNREINFORCED CELLS ONLY.
- PROVIDE ROOF EQUIPMENT FRAME AT ALL ROOF DRAINS AND ROOF HATCH.
- USE BEAM BEARING DETAIL, TYPICAL, UNLESS NOTED.
- EXTEND HSS16X8 2'-0" PAST EDGE OF OPENING.

**STEEL BEAM LEGEND**

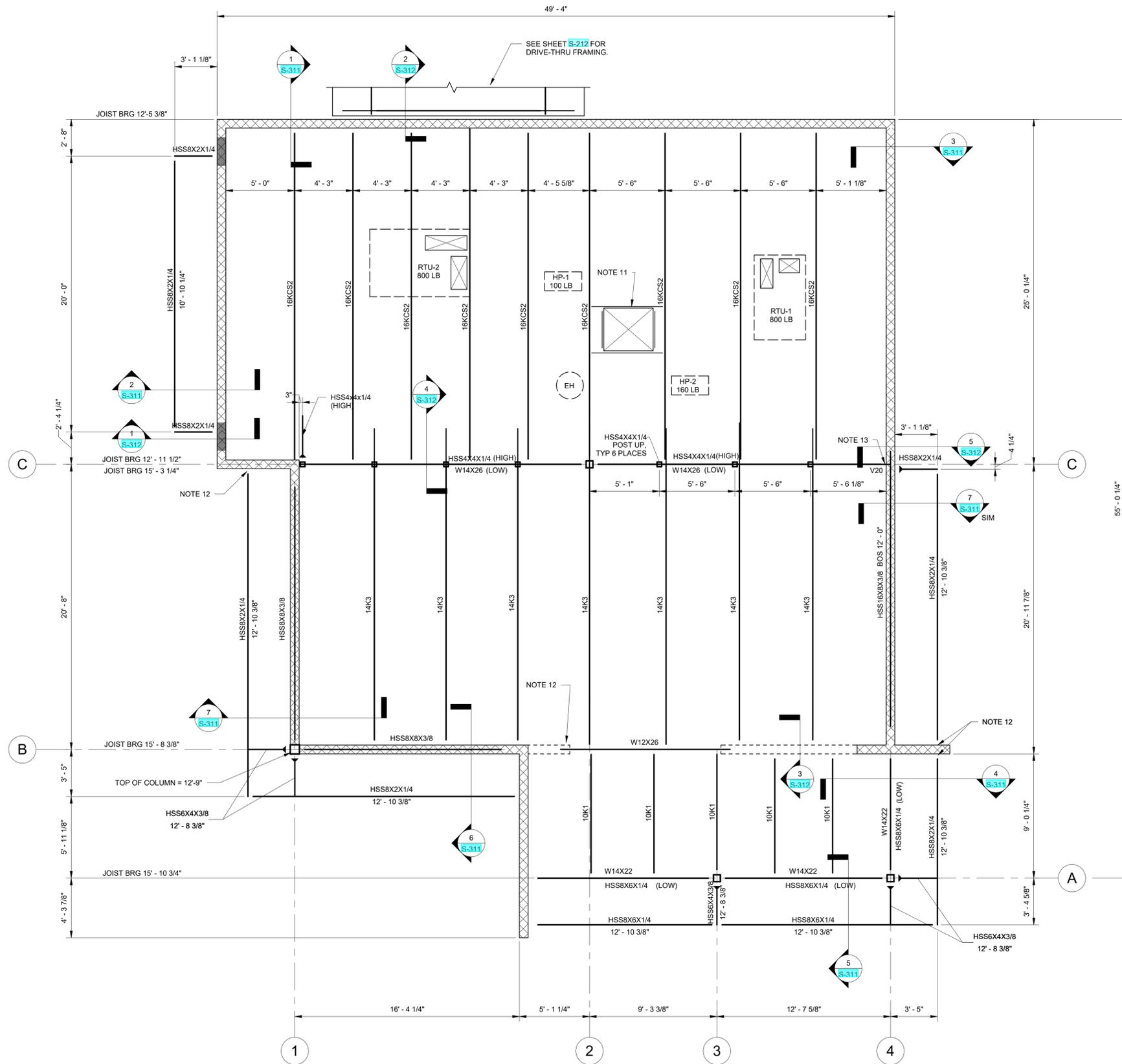
BEAM SIZE  
 # OF HEADED STUDS  
 BEAM CAMBER

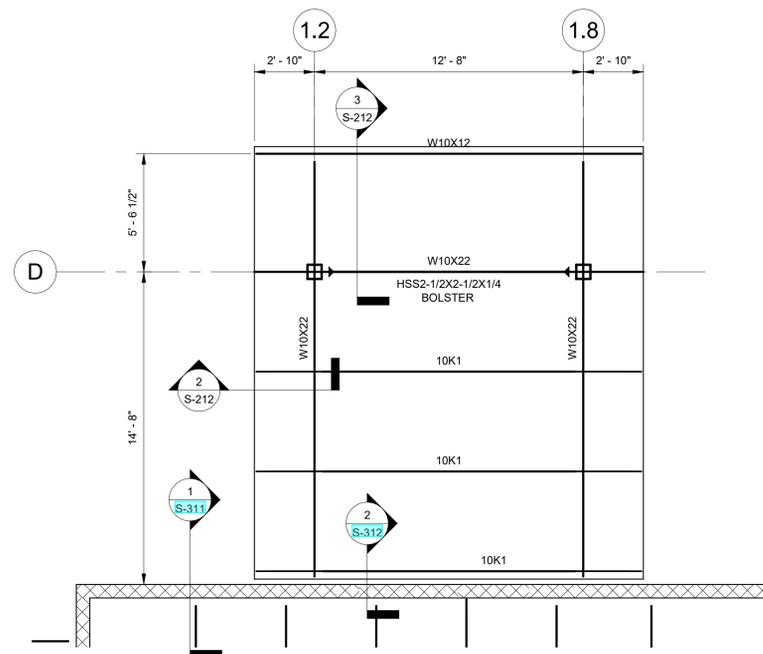
VXX W14x22 [X] c=XX" EPX  
 MXX AXX

EMBED PL - SEE SCHEDULE  
 BEAM AXIAL LOAD (KIPS)  
 BEAM SHEAR REACTION (KIPS)  
 BEAM MOMENT REACTION (KIP-FT)  
 INDICATES MOMENT CONNECTION - SEE TYPICAL DETAILS

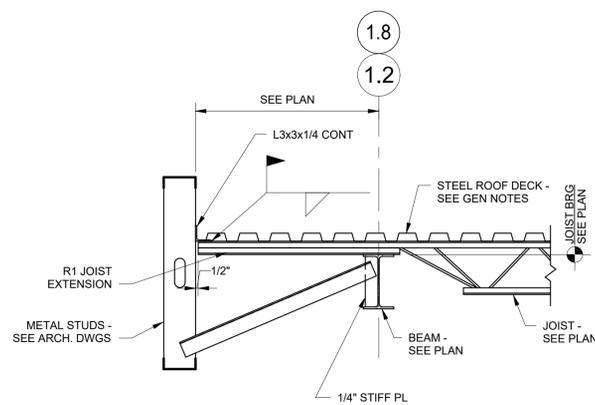
**NOTES:**

- REACTIONS NOTED ON PLAN ARE MAXIMUM LRFD FACTORED REACTIONS. SEE GENERAL NOTES AND TYPICAL DETAILS FOR CONNECTION DESIGN REQUIREMENTS.
- NOTED REACTIONS ACT CONCURRENTLY.
- NOTED MOMENT AND AXIAL REACTIONS ACT IN POSITIVE AND NEGATIVE DIRECTIONS.
- IF NO REACTIONS ARE PROVIDED, CONNECTION ENGINEER TO DESIGN CONNECTIONS FOR LISTED MINIMUM REACTIONS:  
 A. SHEAR: 15 KIPS  
 B. MOMENT: 15 KIP-FT. IF NO MOMENT CONNECTION SYMBOL IS SHOWN, DO NOT DESIGN CONNECTION FOR MOMENT.  
 C. AXIAL: IF NO AXIAL IS PROVIDED, DO NOT DESIGN CONNECTION FOR AXIAL LOAD.
- FOR HEADED STUDS, SEE PLAN NOTES AND TYPICAL DETAILS.
- IF NUMBER OF HEADED STUDS IS NOT PROVIDED, NONE ARE REQUIRED AND THE BEAM IS NON-COMPOSITE.
- IF NO CAMBER IS LISTED, ORIENT NATURAL BEAM CAMBER UPWARD.

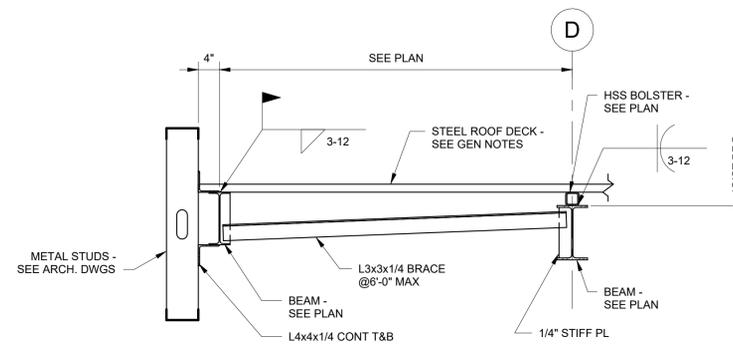




**1** Drive-Thru Roof Framing Plan  
 1/4" = 1'-0"  
 1. TOS (JOIST BEARING) = 12'-3 1/2", UNLESS NOTED.  
 2. FOR PLAN NOTES, SEE SHEET S-211.



**2** Section  
 3/4" = 1'-0"



**3** Section  
 3/4" = 1'-0"

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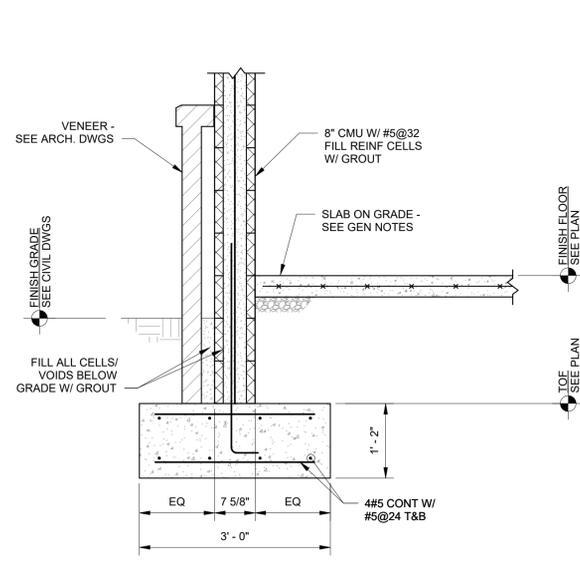
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DATE	08/27/24
DRAWN BY:	Author
CHECKED BY:	Checker

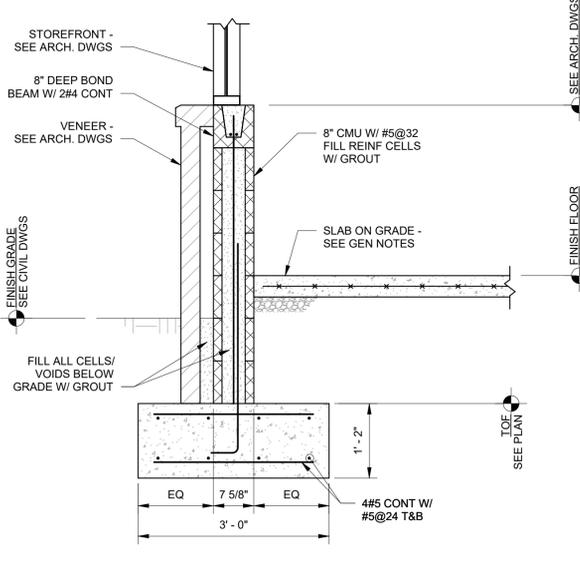
**SHEET TITLE**  
 BRANCH DRIVE-THRU  
 FRAMING PLAN

**SHEET NUMBER**

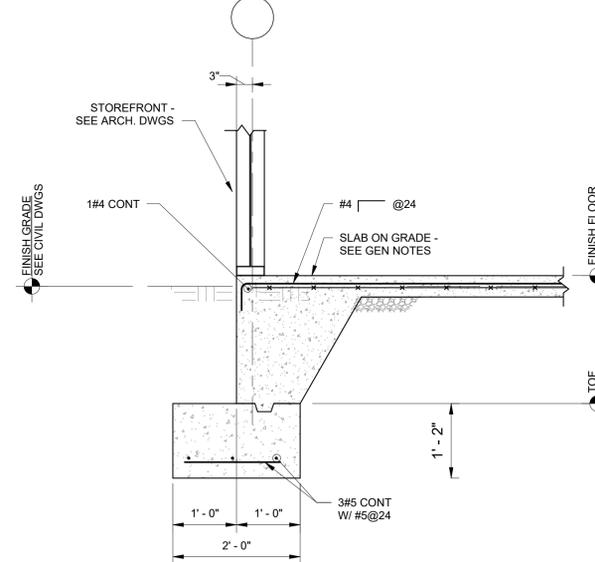
S-212



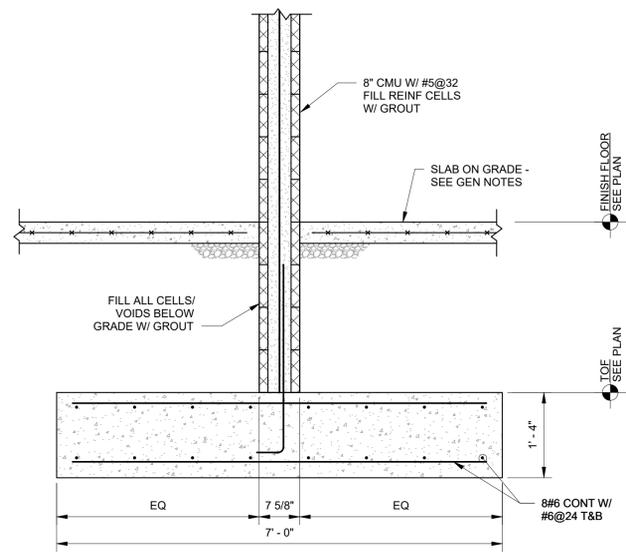
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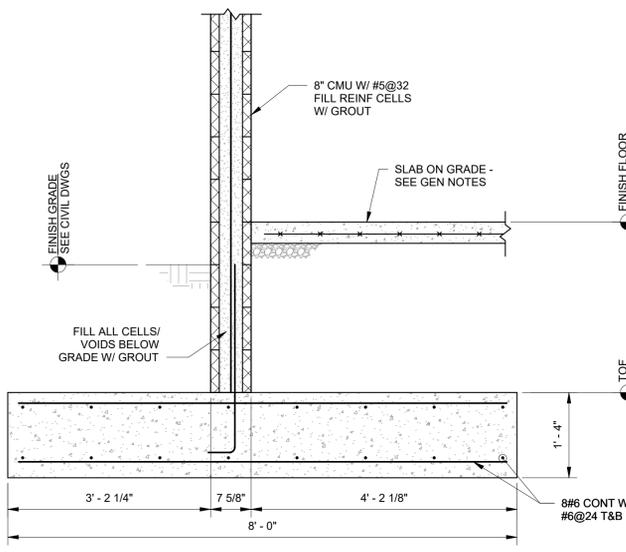
**2 Section**  
 3/4" = 1'-0"



**3 Section**  
 3/4" = 1'-0"



**4 Section**  
 3/4" = 1'-0"



**5 Section**  
 3/4" = 1'-0"

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SHEET TITLE  
**FOUNDATION SECTIONS**  
 SHEET NUMBER  
**S-301**

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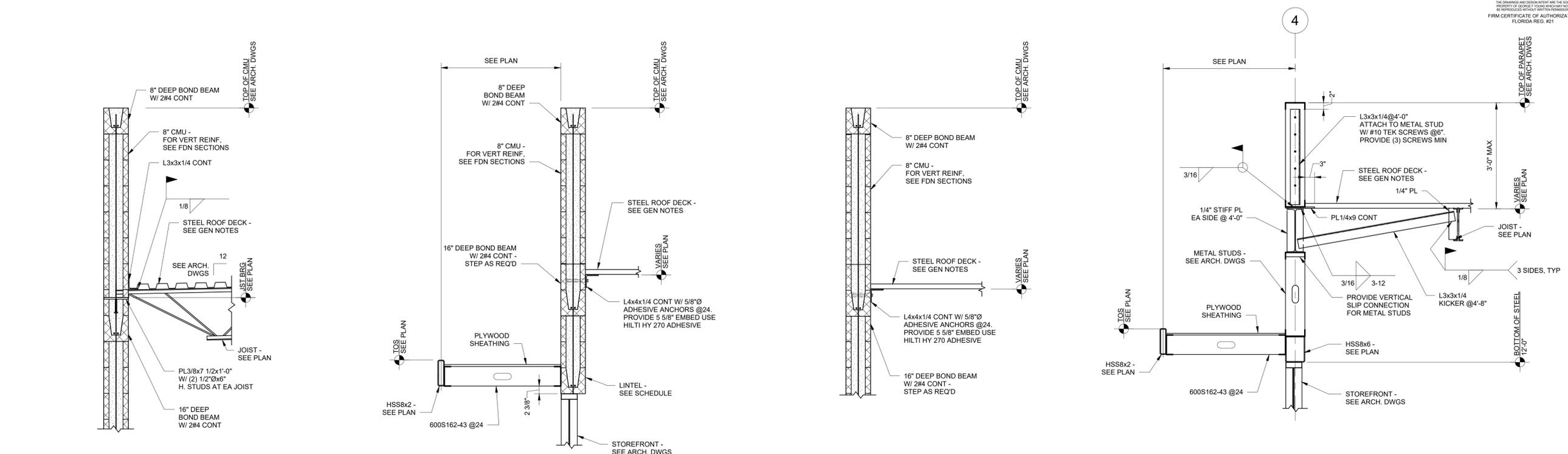
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JOB #	230646
DATE	08/27/24
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CHECKED BY:	JDP

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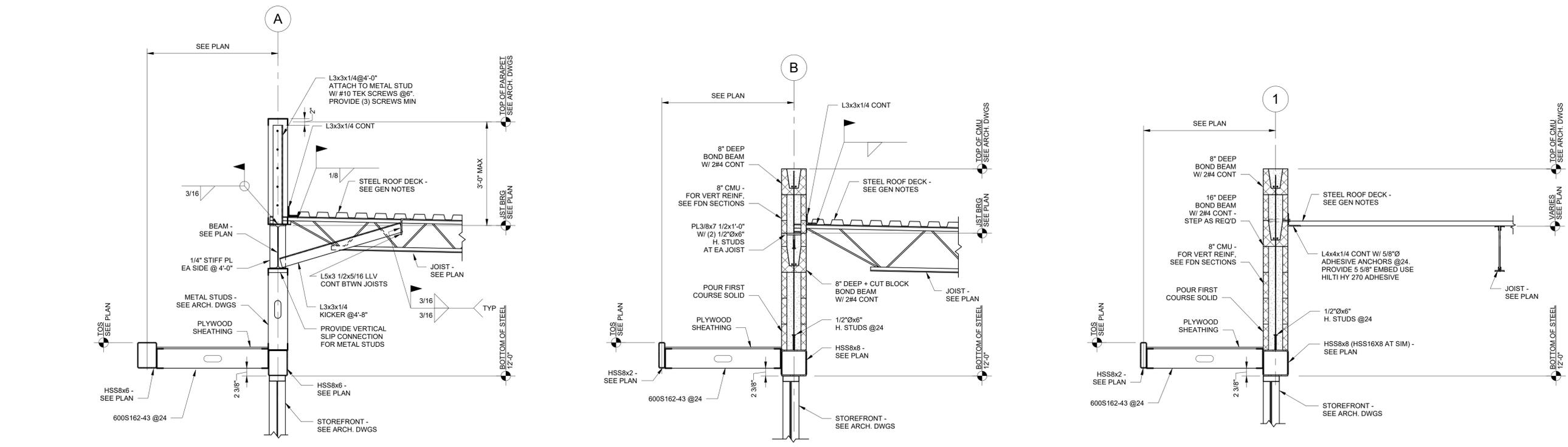
**ROOF SECTIONS**

SHEET NUMBER

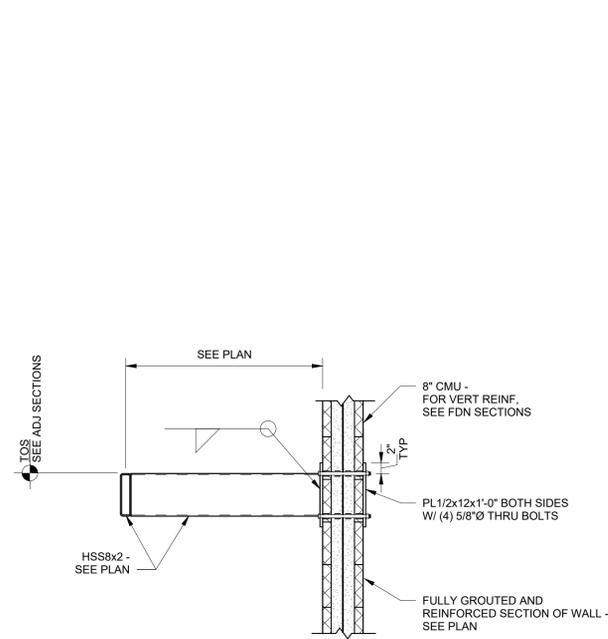
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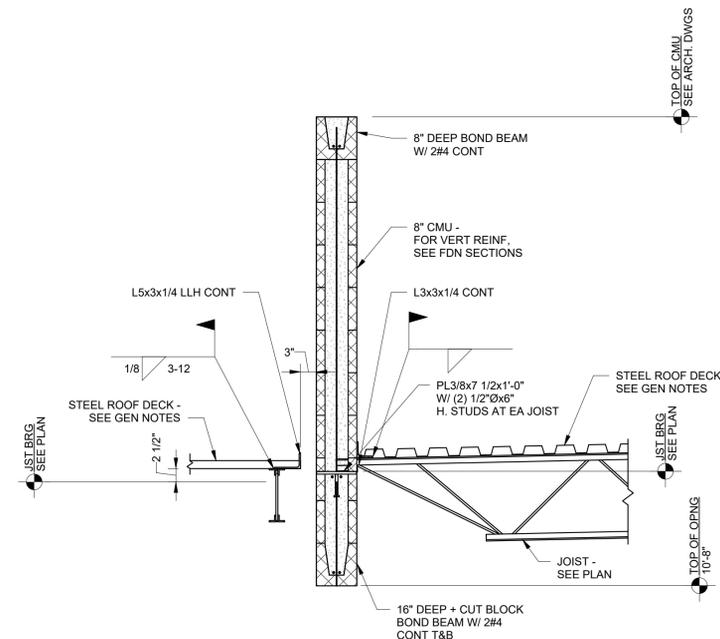
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**2 Section** 3/4" = 1'-0"  
**3 Section** 3/4" = 1'-0"  
**4 Section** 3/4" = 1'-0"



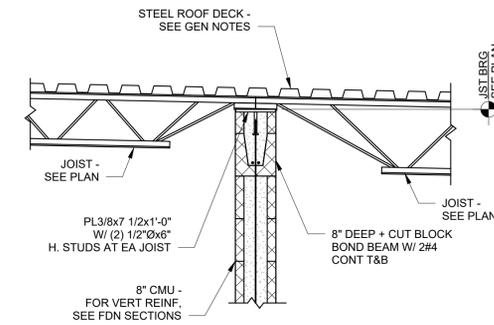
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**6 Section** 3/4" = 1'-0"  
**7 Section** 3/4" = 1'-0"



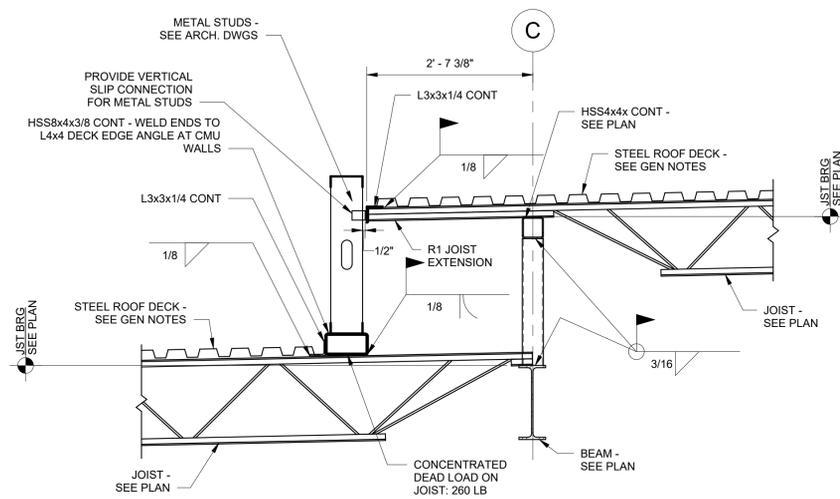
**1 Section**  
 3/4" = 1'-0"



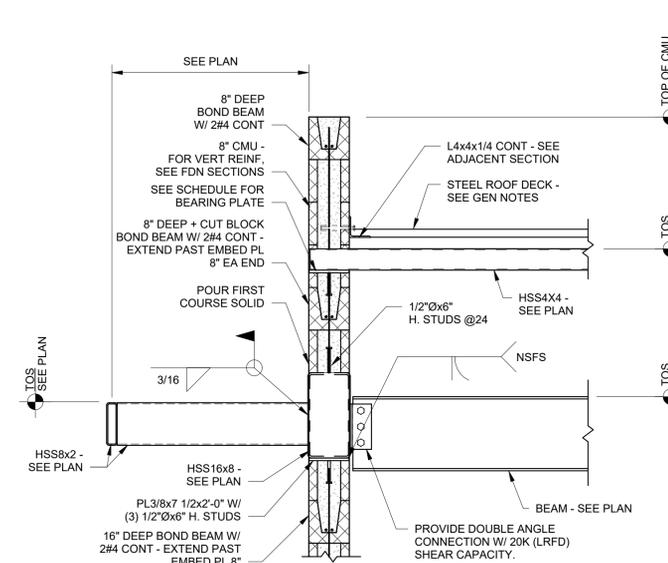
**2 Section**  
 3/4" = 1'-0"



**3 Section**  
 3/4" = 1'-0"



**4 Section**  
 3/4" = 1'-0"



**5 Section**  
 3/4" = 1'-0"

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**SHEET TITLE**  
 ROOF SECTIONS  
**SHEET NUMBER**  
 S-312

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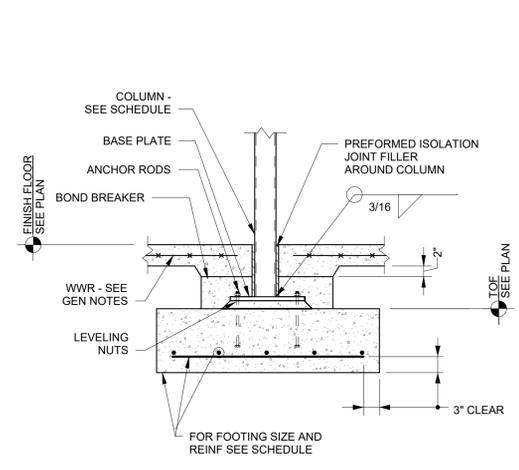
PROJECT INFORMATION BLOCK	
JOB #	230646
DATE	08/27/24
DRAWN BY:	SSC
CHECKED BY:	JDP

SHEET TITLE

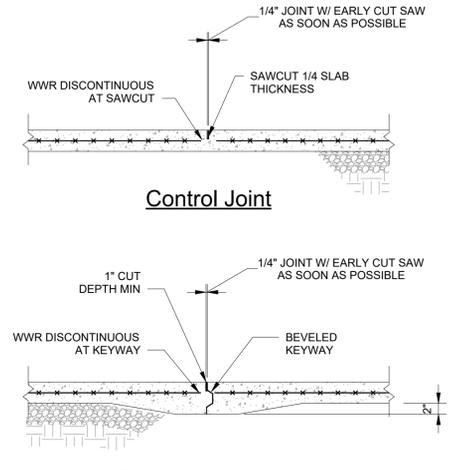
TYPICAL DETAILS

SHEET NUMBER

S-401



**1** HSS Column Base and Footing Detail  
 N.T.S.

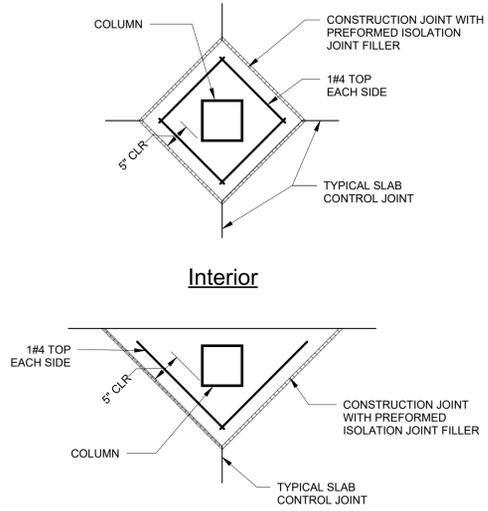


**2** Slab Control Joint Details  
 N.T.S.

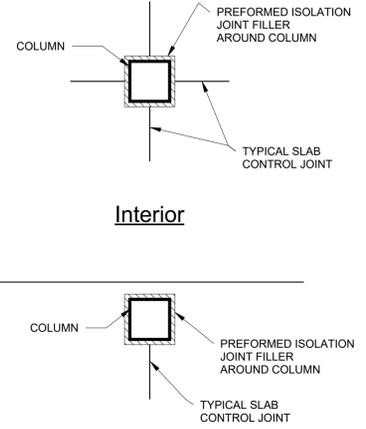
BAR SIZE	f <sub>c</sub> = 3000	
	TOP BARS	OTHER BARS
#3	28"	22"
#4	37"	29"
#5	47"	36"
#6	56"	43"
#7	81"	63"
#8	93"	72"

NOTES:  
 1. TOP BARS ARE HORIZONTAL REINFORCEMENT WITH MORE THAN 12" OF CONCRETE CAST BELOW THE REINFORCEMENT.

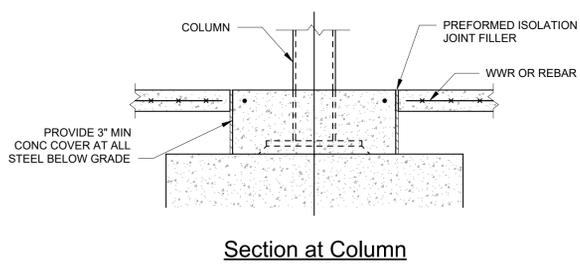
**3** Concrete Tension Lap Splice Lengths  
 N.T.S.



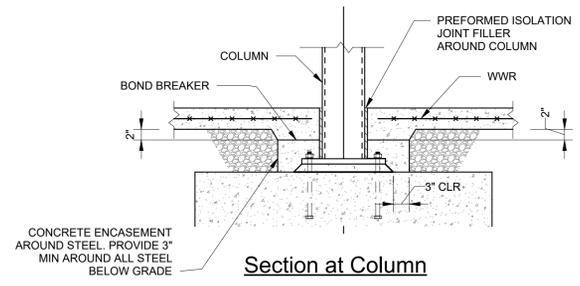
**Interior**  
 Isolation Joint Detail



**Exterior**  
 Isolation Joint Detail

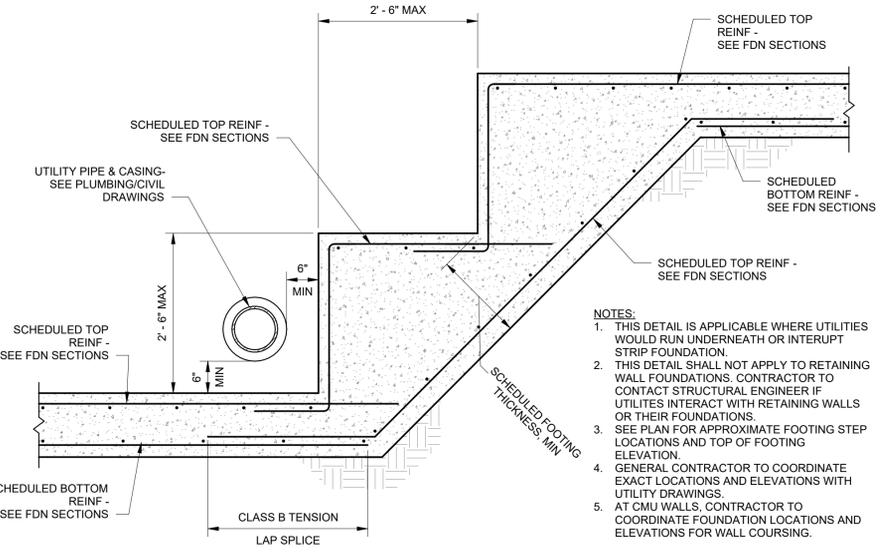


Section at Column

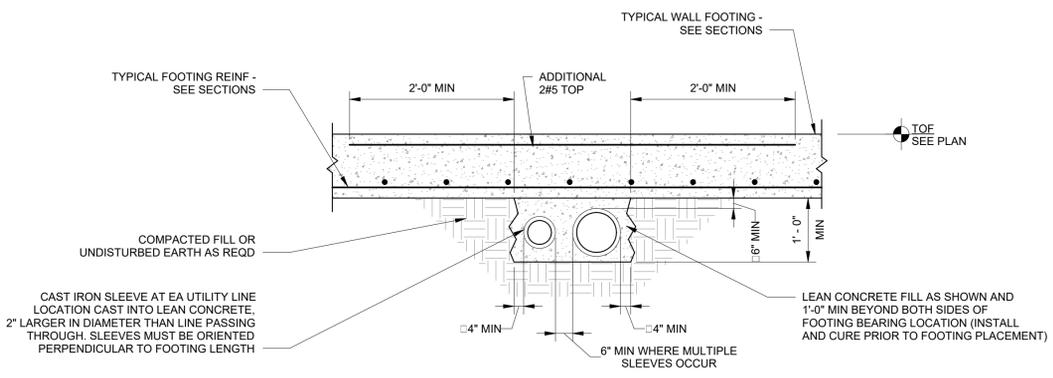


Section at Column

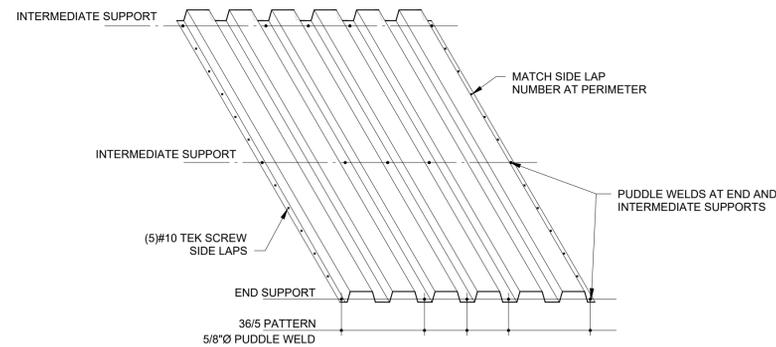
**4** Column/Slab-Joint Details  
 N.T.S.  
 CONTRACTOR'S OPTION TO USE EITHER JOINT DETAIL



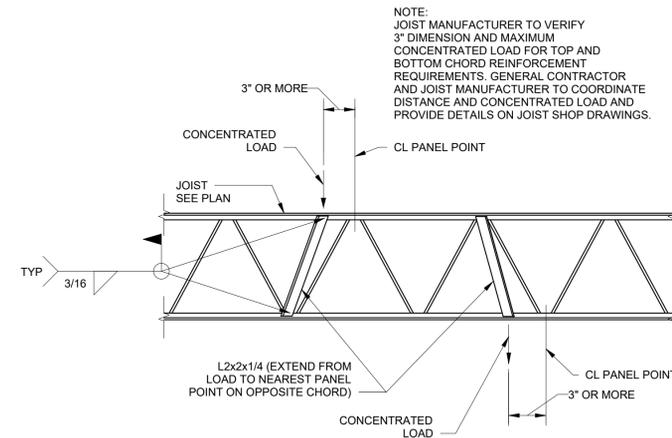
**5** Stepped Strip Footing Detail  
 N.T.S.



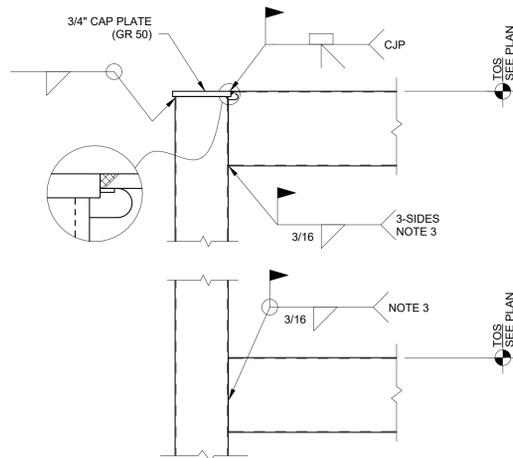
**6** Typical Detail for Utilities Passing Below Wall Footings  
 N.T.S.  
 1. CONTRACTOR'S OPTION TO STEP FOOTINGS BELOW UTILITIES IN LIEU OF THIS DETAIL.  
 2. COORDINATE UTILITY LOCATIONS W/ CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.  
 3. UTILITIES SHALL NOT PASS BELOW COLUMN FOOTINGS.



**1** 1 1/2" Type B Roof Deck Attachment Detail  
 N.T.S.

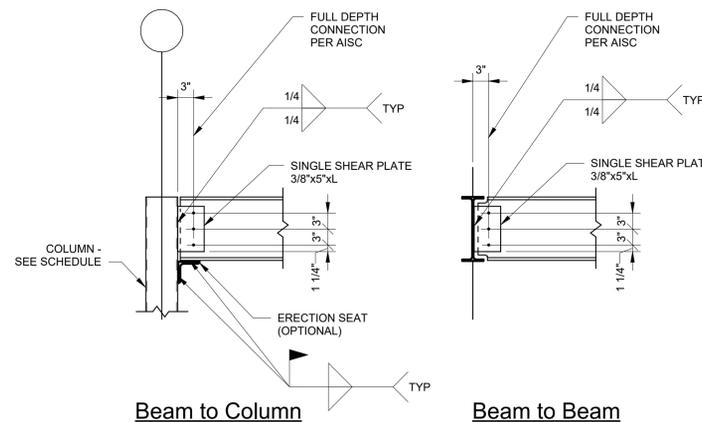


**2** Joist Reinforcement Detail  
 N.T.S.

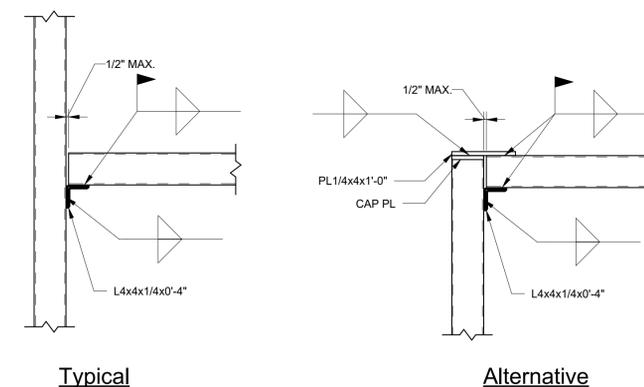


**3** HSS Beam - HSS Column Moment Connection  
 N.T.S.

- NOTES:  
 1. SEE PLAN FOR MOMENT CONNECTION LOCATIONS.  
 2. INCREASE FILLET WELD SIZE FOR GAP.  
 MAXIMUM GAP PERMITTED = 3/16"  
 3. OPTIONAL GROOVE WELD AS REQUIRED BY LOADING.  
 USE FLARE-BEVEL GROOVE WELD (BTC-P10) ON VERTICAL SIDES IF WIDTHS OF BEAM AND COLUMN ARE EQUAL.  
 4. CONNECTION DESIGNER TO CHECK CONCENTRATED LOADS ON SUPPORTING MEMBER & SIZE WELDS.



**4** Beam/Column and Beam/Beam Connection Details  
 N.T.S.



**6** HSS Beam - HSS Column Connection  
 N.T.S.

- NOTES:  
 1. AT HSS BEAMS DEEPER THAN 8", PROVIDE L4x4 AT TOP AND BOTTOM. IF COLUMN DOES NOT EXTEND ABOVE BEAM, USE ALTERNATIVE.

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SHEET NUMBER  
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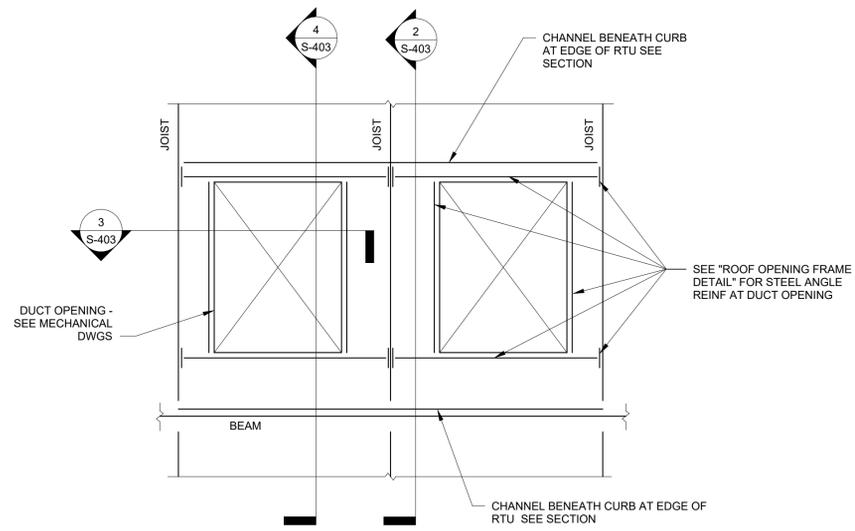
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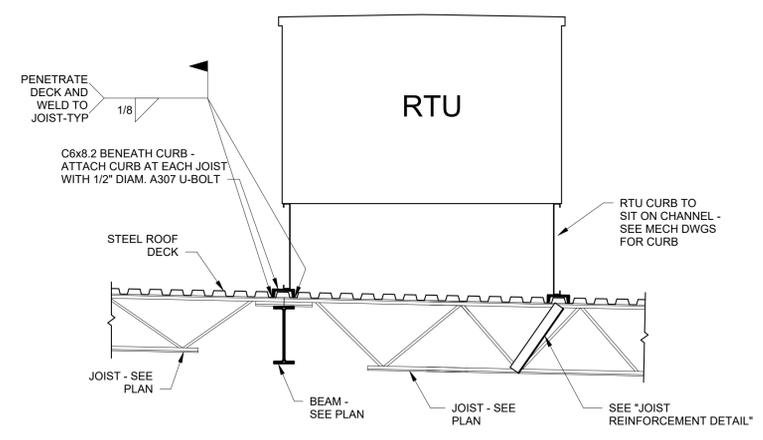
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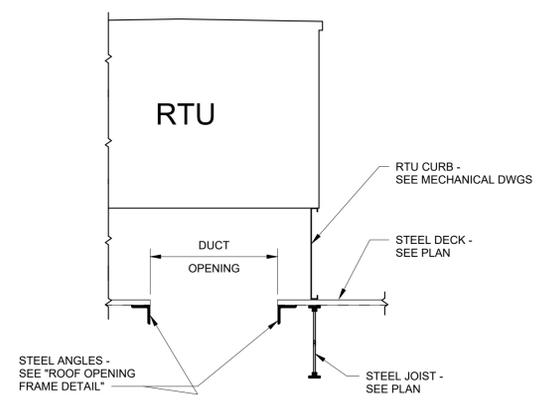
**S-403**



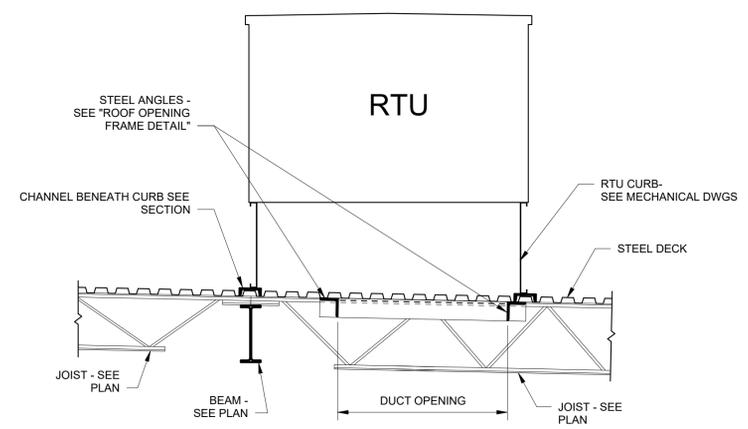
**1** Enlarged Roof Plan at RTU  
 N.T.S.



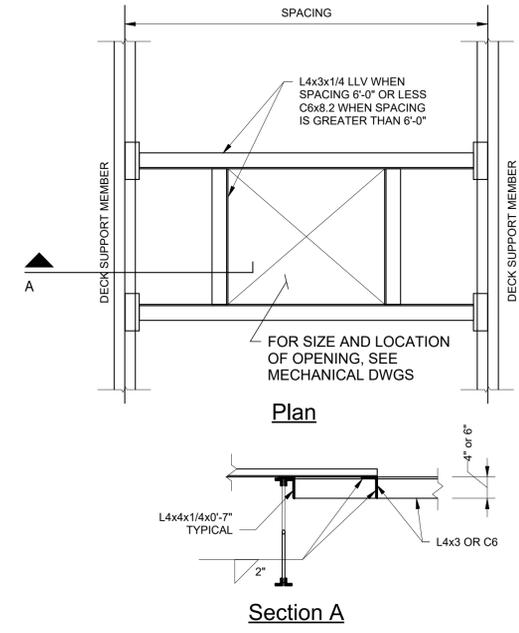
**2** Section  
 N.T.S.



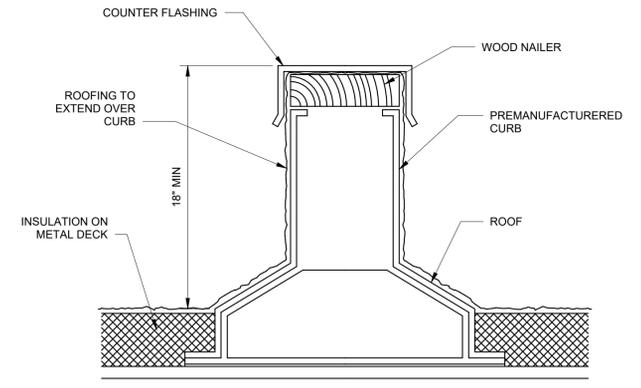
**3** Section  
 N.T.S.



**4** Section  
 N.T.S.

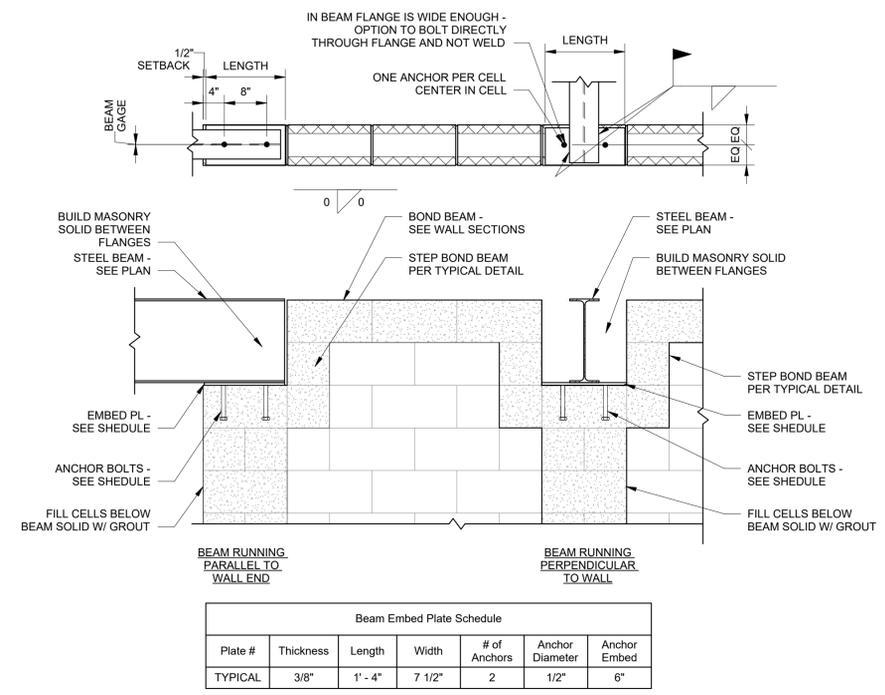


**5** Roof Opening Frame Detail  
 N.T.S.  
 AT ALL OPENINGS IN ROOF LARGER THAN 8"

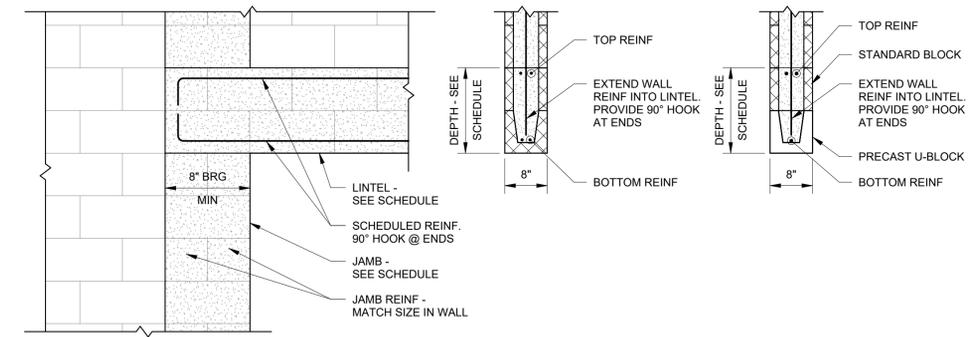


**NOTES:**  
 1. SECURE CURB TO ROOF AS NOTED BELOW. IF ROOF CONSTRUCTION VARIES, CONTACT STRUCTURAL ENGINEER.  
 A. AT METAL ROOF DECK, USE #10 TEK SCREWS @ 6" SPACING.

**6** Equipment Support Curb  
 N.T.S.

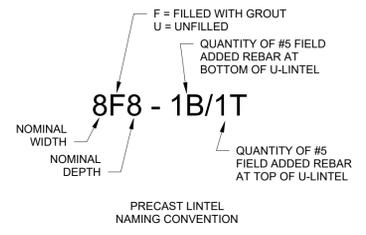


**1** Beam Embed Plate Schedule  
 N.T.S.

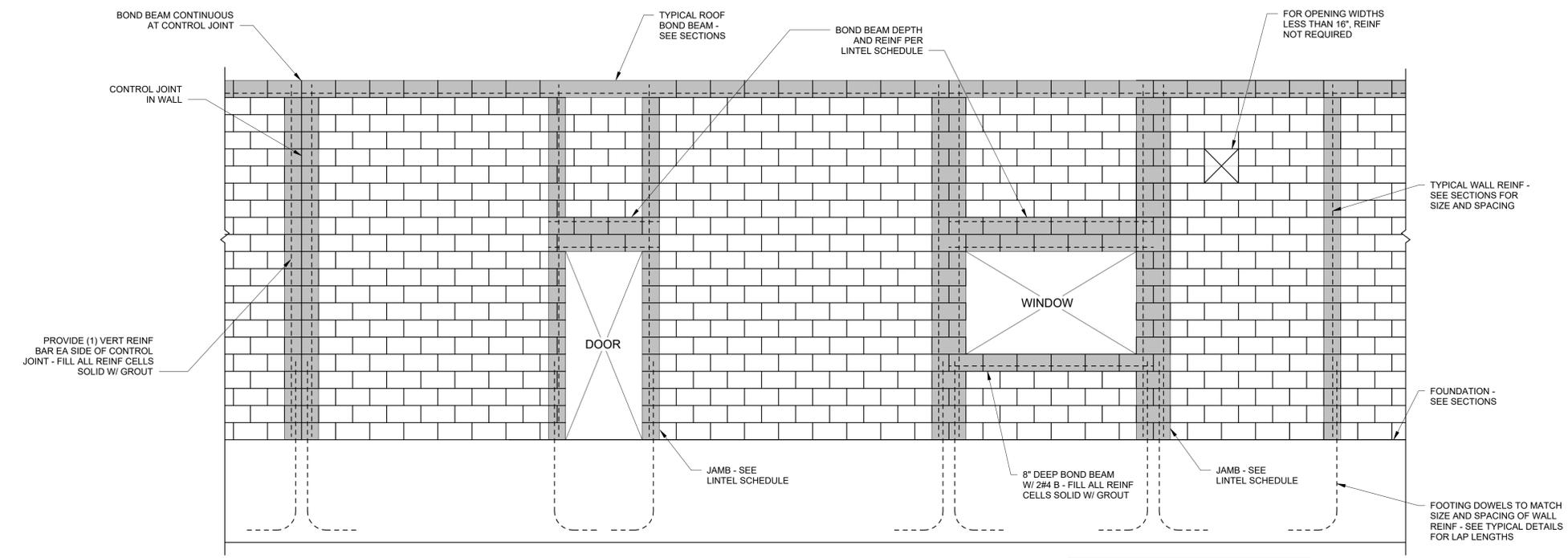


CMU Lintel Schedule						Precast Lintel
Opening Width	Wall Thickness	Lintel Depth	Bottom Reinf	Top Reinf	Jamb	
6'-0"	8"	8"	2#4	---	2 Cells	---
12'-0"	8"	24"	*	*	3 Cells	8F24-1B/1T

\* USE REINFORCEMENT AS INDICATED BY SPECIFIED PRECAST LINTEL.



**2** Masonry Lintel Detail  
 N.T.S.



RUNNING BOND IS SHOWN IN THIS DETAIL FOR ILLUSTRATION PURPOSES ONLY. REFER TO GENERAL NOTES FOR JOB SPECIFIC BOND PATTERN.

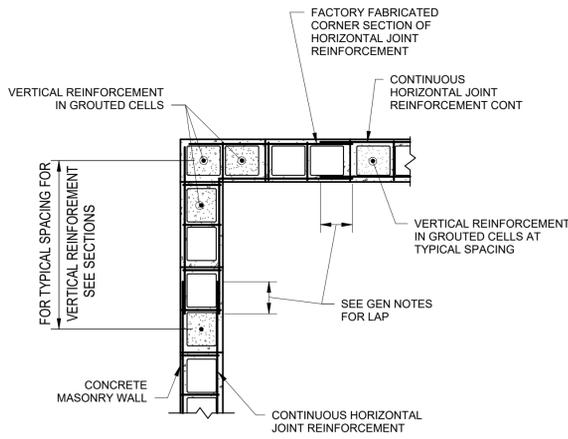
**3** Door and Window Opening Reinforcement at Load Bearing Masonry Walls  
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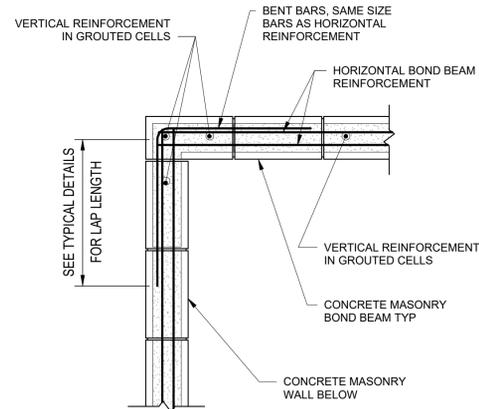
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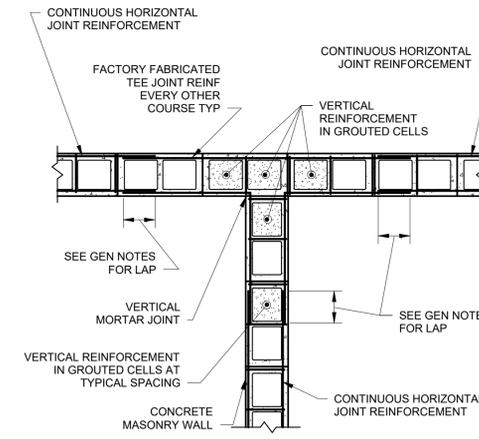
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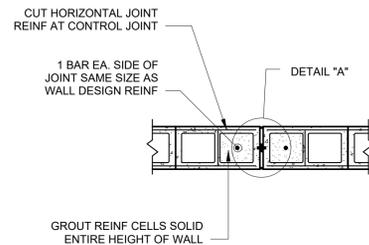
**1** Plan Showing Joint Reinforcement at Wall Corner  
 N.T.S.



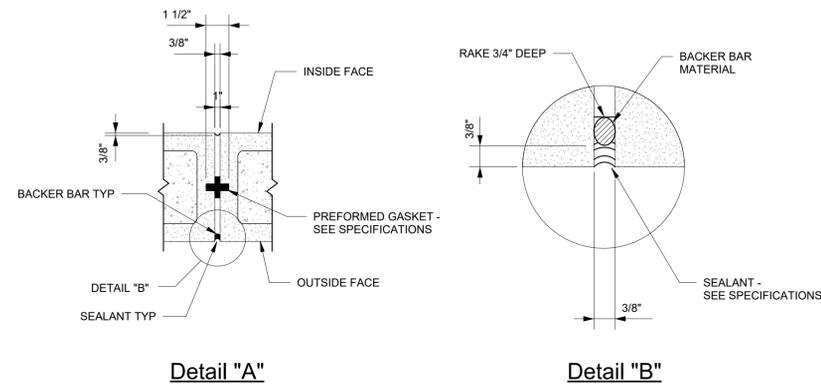
**2** Plan Showing Bond Beam Reinforcement at Wall Corner  
 N.T.S.



**3** Plan Showing Joint Reinforcement at Structural Wall Intersection  
 N.T.S.



NOTE:  
 SEE ARCHITECTURAL DRAWINGS FOR CONTROL JOINT LOCATIONS. CONTROL JOINT SPACING NOT TO EXCEED 25'-0" OR 1.5 TIMES THE WALL HEIGHT, WHICHEVER IS LESS.



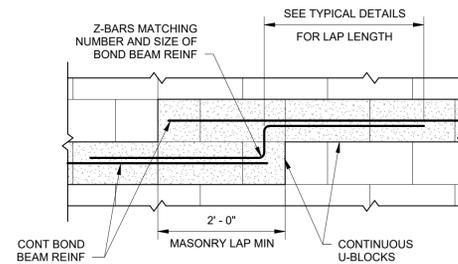
**5** Masonry Control Joint  
 N.T.S.

**Masonry Tension Lap Splice Lengths**

BAR SIZE	f <sub>m</sub> = 2000
#4	18"
#5	20"
#6	38"
#7	52"

NOTES:  
 1. THIS TABLE CONTAINS DEVELOPMENT AND SPLICE LENGTHS FOR REINFORCEMENT IN CMU ACCORDING TO ACI 530-13/TMS 402-13/ASCE 5-13.  
 2. FOR DEVELOPMENT AND SPLICE LENGTHS OF EPOXY COATED BARS, MULTIPLY TABULATED VALUES BY 1.5.  
 3. WHERE VERTICAL BAR IS INTERRUPTED BY A BEARING PLATE OR OTHER OBSTRUCTION, PROVIDE BACK DOWEL MATCHING VERTICAL REINFORCEMENT W/ 8" MAXIMUM OFFSET.

**4** Masonry Tension Lap Splice Lengths  
 N.T.S.



**6** Bond Beam Step Detail  
 N.T.S.

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